

**UNIVERSIDAD POLITECNICA DE VALENCIA**  
**DEPARTAMENTO DE INGENIERIA MECANICA Y DE MATERIALES**

**ELEMENTOS FINITOS**  
**(E.T.S.I.I.V)**

**MODELADO POR ELEMENTOS FINITOS**  
**LECCION 2.- MALLADO, CONDICIONES DE CONTORNO Y CARGAS**

**J. L. OLIVER**  
**Dr. Ingeniero Industrial**

**Valencia, 2005**

## Topics in Chapter 8



**General Modeling Rules**

**Finite Element Mesh Layouts**

**Distributed Loads**

**Displacement BCs**

**suppressing rigid body motions**  
**taking advantage of symmetry and antisymmetry**

## General FEM Modeling Rules



- **Use the simplest elements that will do the job**
- ***Never, never, never* use complicated or special elements unless you are absolutely sure of what you are doing**
- **Use the coarsest mesh that will capture the dominant behavior of the physical model, particularly in *design* situations**

**3 word summary: *Keep It Simple***

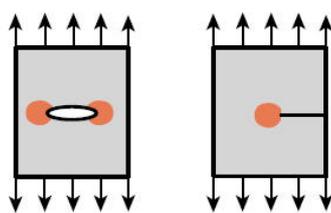
## Another Justification for Simplicity

**In product design situations  
several FEM models of increasing refinement  
will be set up as design evolves**

**Ergo, do not overkill at the beginning**

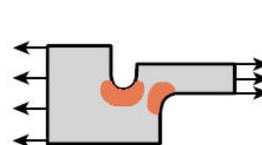


## Where Finer Meshes Should be Used

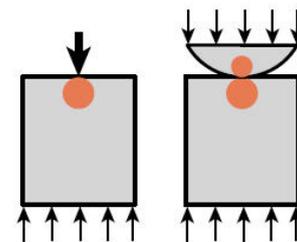
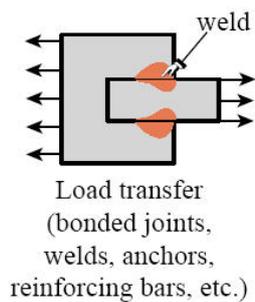
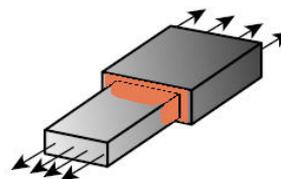
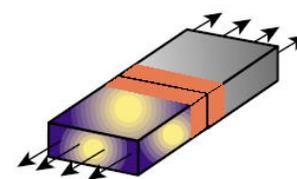


Cutouts

Cracks

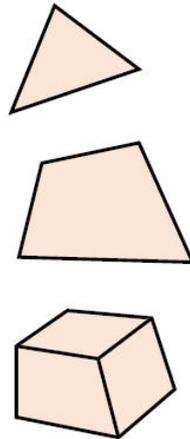


entrant corners

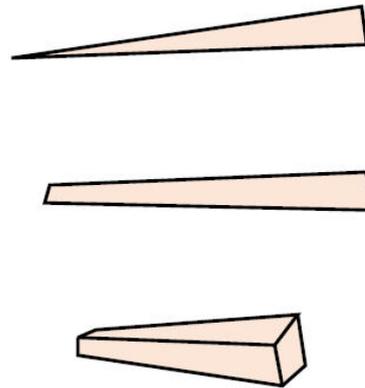
Vicinity of concentrated (point)  
loads, and sharp contact areasLoad transfer  
(bonded joints,  
welds, anchors,  
reinforcing bars, etc.)Abrupt thickness  
changesMaterial  
interfaces

## Avoid 2D/3D Elements of Bad Aspect Ratio

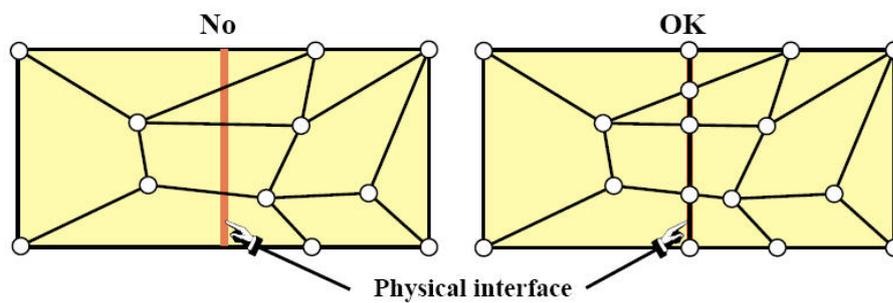
Good



Bad



## Elements Must Not Cross Interfaces



## Element Geometry Preferences

Other things being equal, prefer

**in 2D: Quadrilaterals over Triangles**

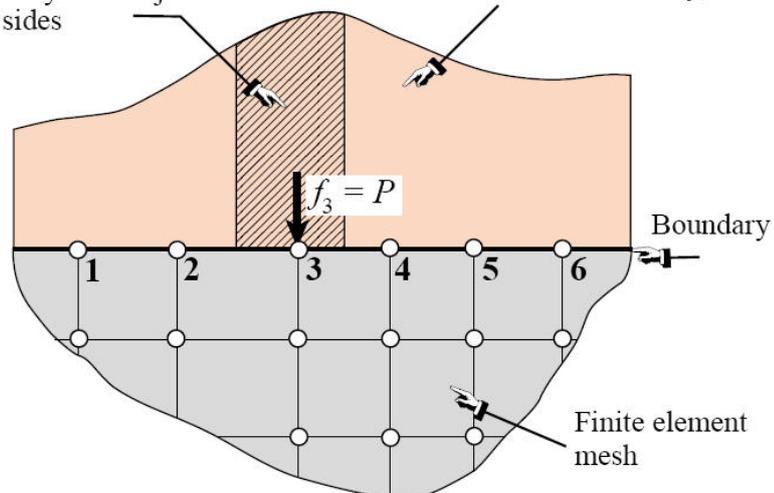
**in 3D: Bricks over Wedges  
Wedges over Tetrahedra**

(Elements do not file discrimination suits)

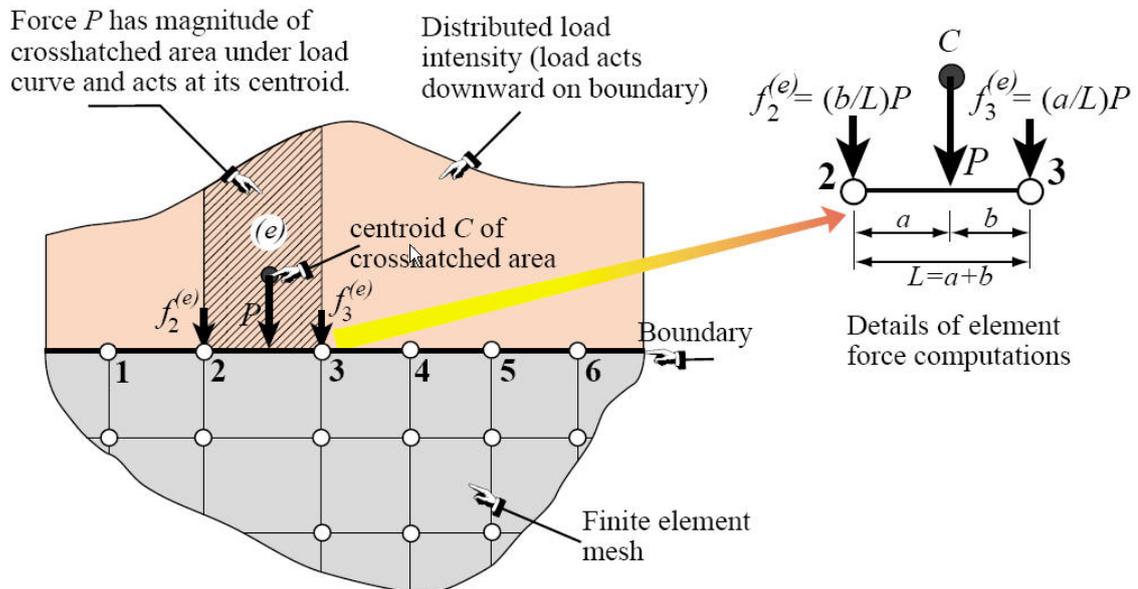
## Node by Node (NbN) Distributed Load Lumping

Nodal force  $f_3$  at 3 is set to  $P$ , the magnitude of the crosshatched area under the load curve. This area goes halfway over adjacent element sides

Distributed load intensity (load acts downward on boundary)



## Element by Element (EbE) Distributed Load Lumping



## Boundary Conditions (BCs)

→ The most difficult topic for FEM  
program users

Two types

Essential

Natural

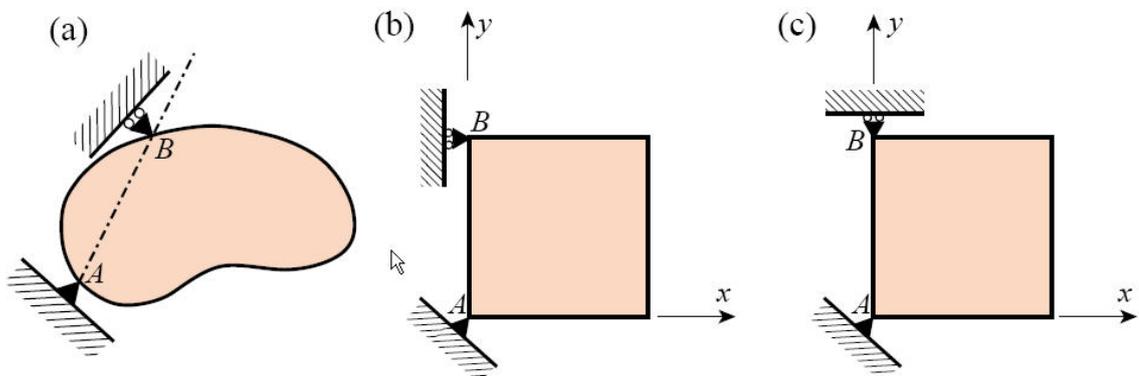
## Boundary Conditions

### Essential vs. Natural

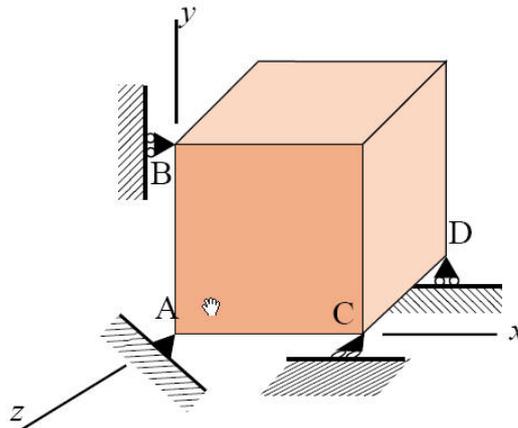
#### Recipe:

1. If a BC involves one or more DOF in a *direct way*, it is *essential* and goes to the **Left Hand Side (LHS)** of  $Ku = f$
2. Otherwise it is *natural* and goes to the **Right Hand Side (RHS)** of  $Ku = f$

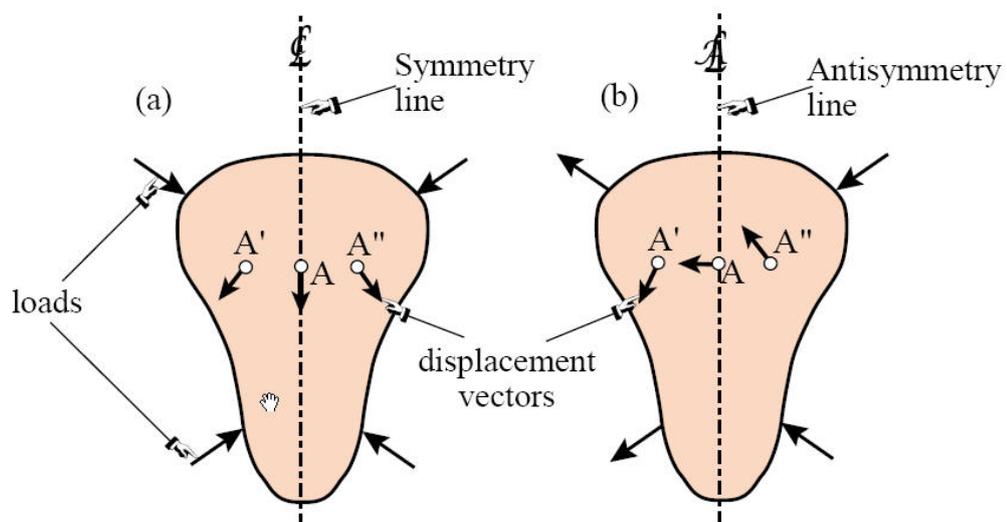
## Minimum Support Conditions to Suppress Rigid Body Motions in 2D



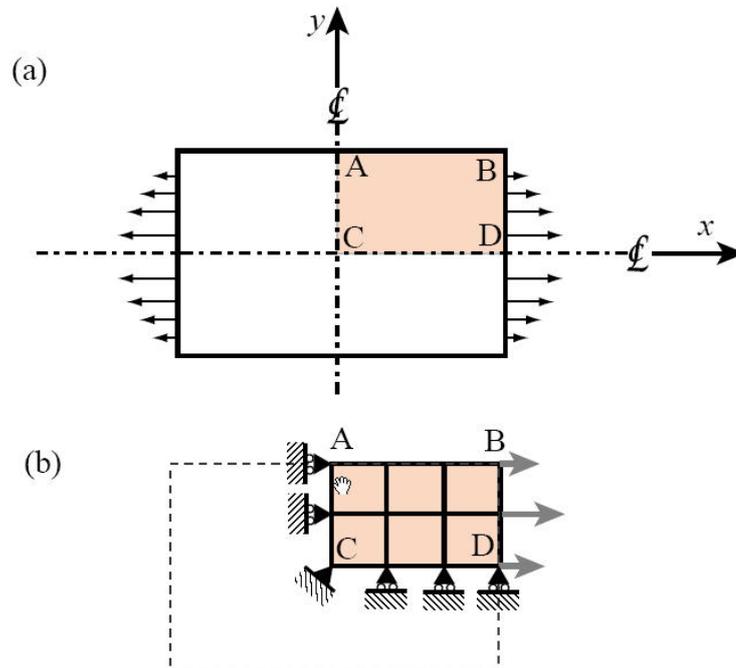
## Minimum Support Conditions to Supress Rigid Body Motions in 3D



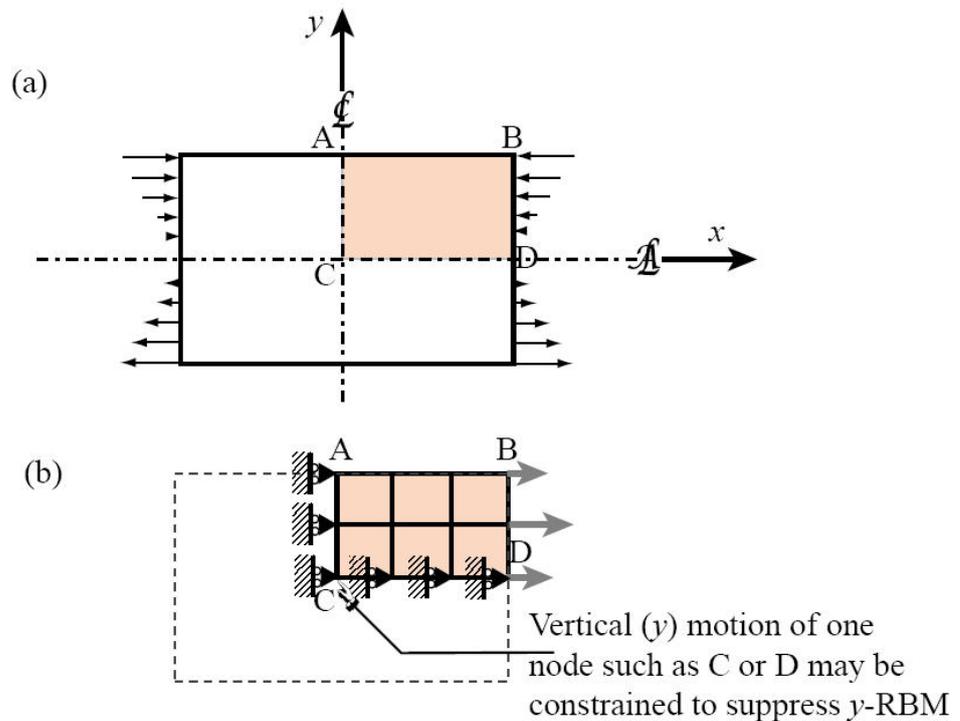
## Visualizing Symmetry and Antisymmetry Conditions in 2D

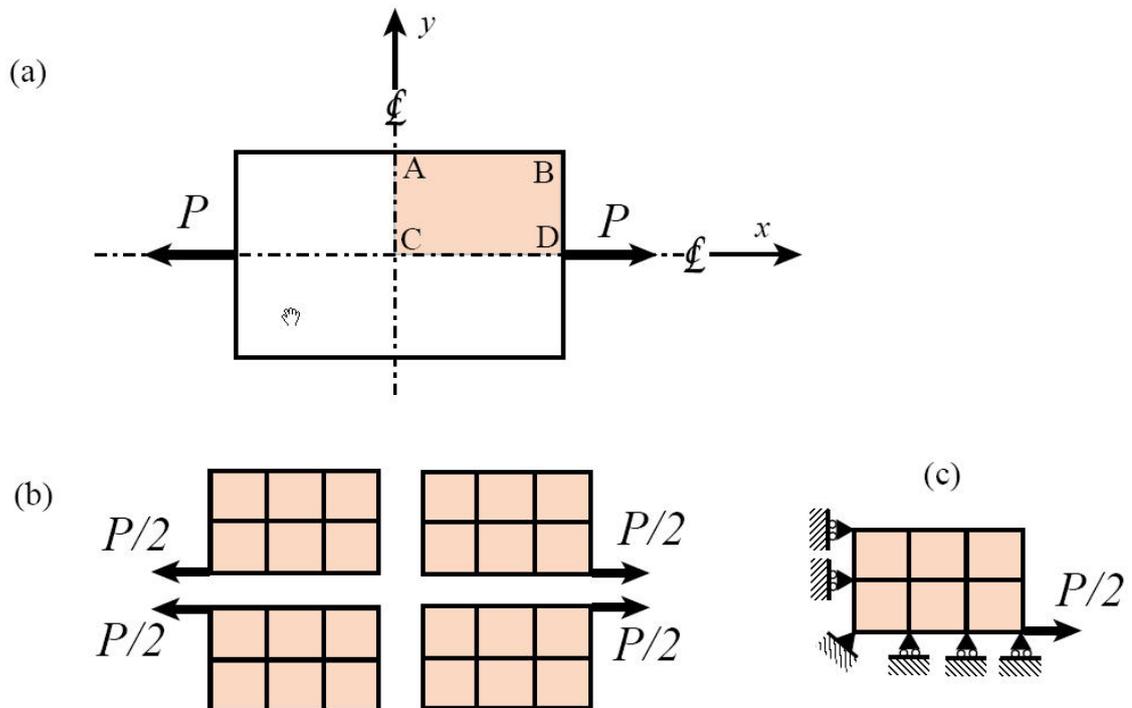


## Example of Application of Symmetry BCs

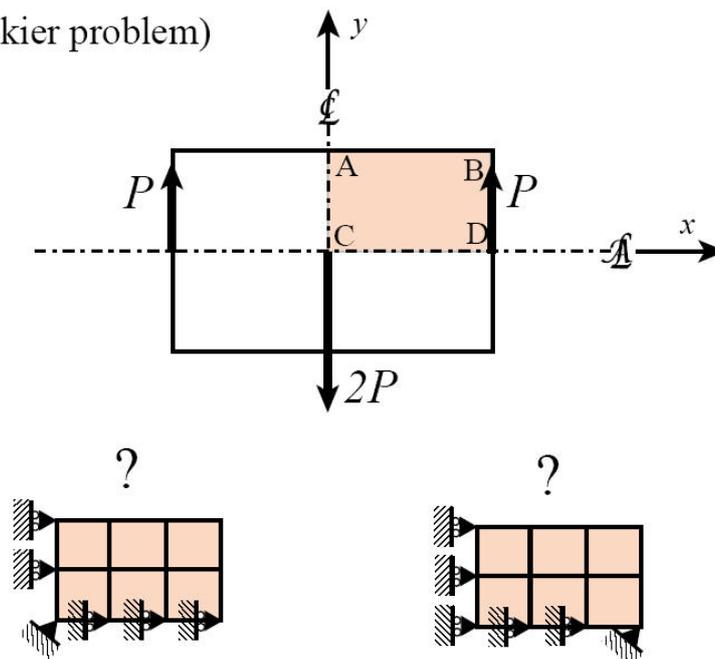


## Example of Application of Antisymmetry BCs



**"Breaking Up" Point Loads at Symmetry BCs****"Breaking Up" Point Loads at Antisymmetry BCs**

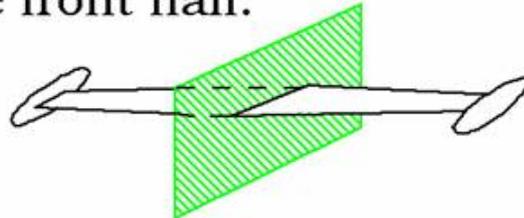
(A trickier problem)



# SYMMETRIES

## A. REFLECTIVE SYMMETRY

A body has a reflective plane if the half of the body behind the plane is identical to a mirror image of the front half.

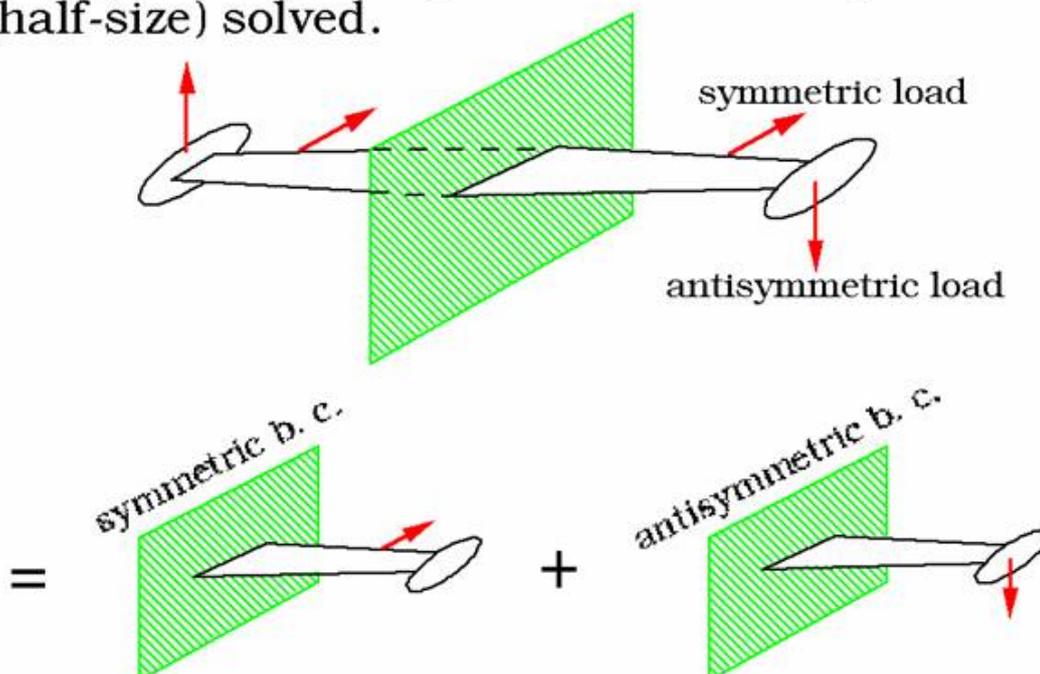


In structural mechanics, one can exploit the symmetry if one has symmetry of:

geometry  
material

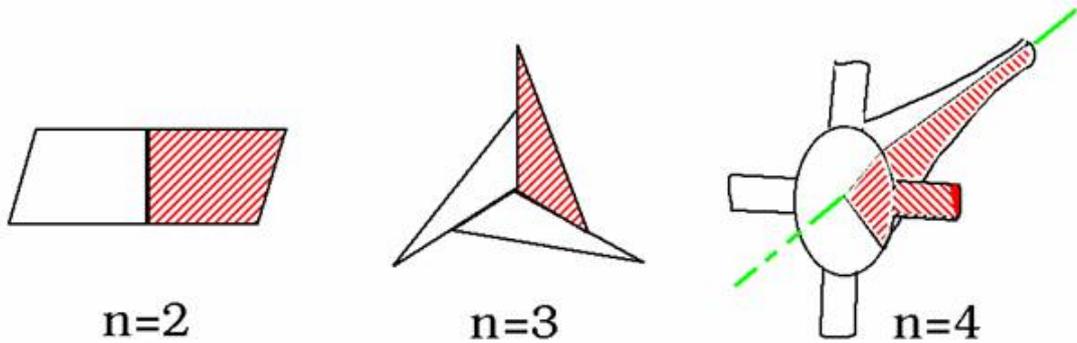
displacement boundary conditions

It is not necessary for the loads to be symmetric. They can be decomposed into symmetric and antisymmetric components, and two problems (half-size) solved.



## B. ROTATIONAL SYMMETRY

The body has a straight axis of symmetry. A fundamental region can generate the entire body by  $n$  replications, equally spaced about the axis:



The NASTRAN<sup>®</sup> codes have a strong rotational (cyclic) symmetry capability. (10)

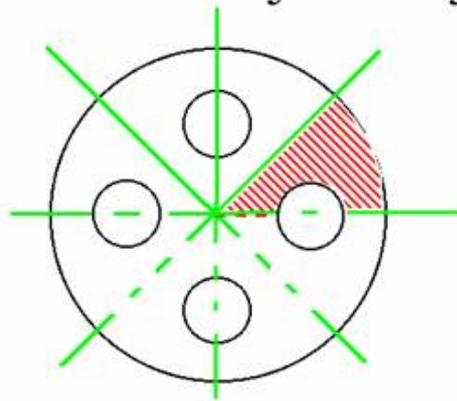
## C. DIHEDRAL SYMMETRY

(11)

Combine rotational and reflective symmetry.

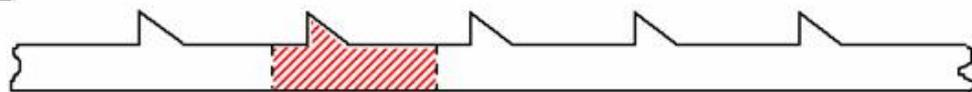
One only needs to model half of one cyclic region.

There are multiple reflective planes.



## D. TRANSLATIONAL SYMMETRY

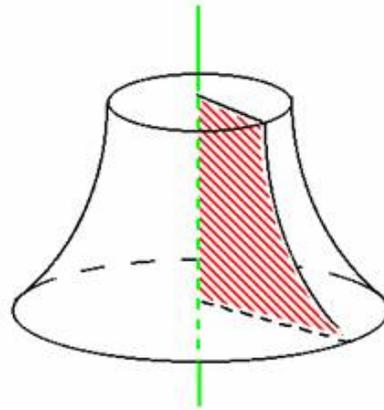
The fundamental pattern is repeated on a straight line:



## E. AXISYMMETRY

A subcase of rotational (cyclic) symmetry:

- straight axis of rotation.
- body is swept out by rotation of one segment.
- lathe pieces are examples.
- can have skew due to winding process for shells, and material anisotropy.

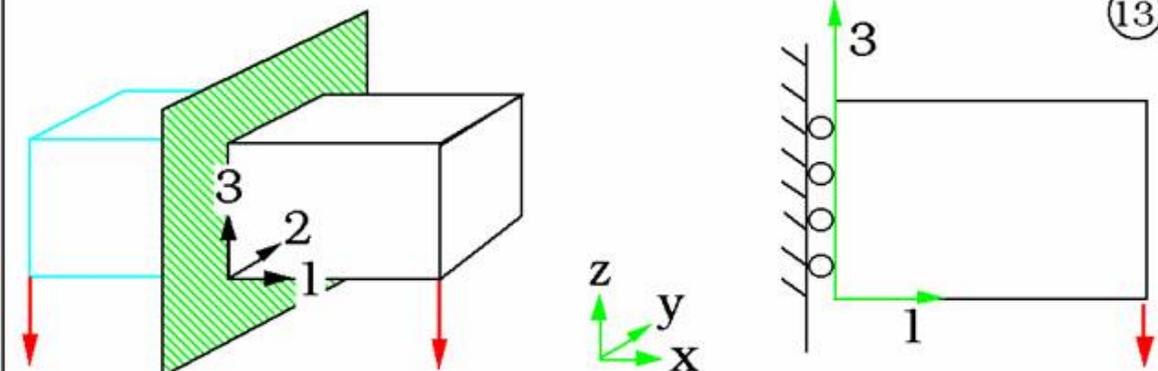


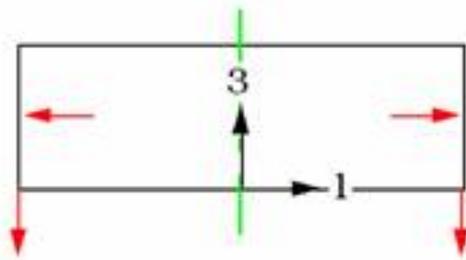
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## BOUNDARY CONDITIONS FOR REFLECTIVE SYMMETRY

### A. "ELASTICITY" ELEMENTS

Consider elements that have only translational degrees of freedom (elasticity elements).



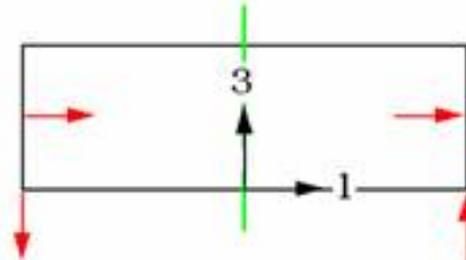


Symmetric B. C.:

$$T_1 = 0$$

$$F_2 = 0$$

$$F_3 = 0$$



Antisymmetric B. C.:

$$F_1 = 0$$

$$T_2 = 0$$

$$T_3 = 0$$

It is easiest to find the antisymmetric conditions as the complement of the symmetric case.

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## RIGID BODY MODES

### A. DEFINITION

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A rigid body mode is a displacement field which causes no strain energy. An elastic body undergoes neither direct nor shear strain.

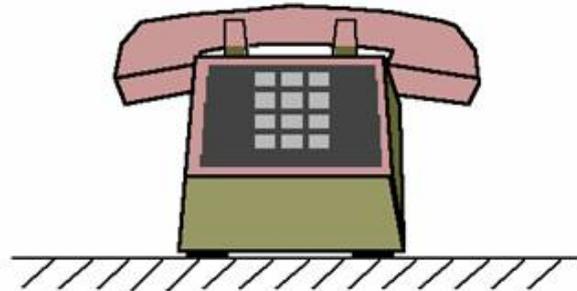
In contrast, straining modes cause strain energy to be developed. Straining modes can, however, include some rigid body motion.

Elastic bodies in three dimensions have 6 rigid body modes, 3 translations and 3 rotations:

surge (skid)	roll
sideslip	pitch
plunge	yaw



Bodies constrained to move in 2-D, such as a desk telephone, have only 3 rigid body modes.



There are two translations and one rotation.

If the receiver falls off, one has a "mechanism."

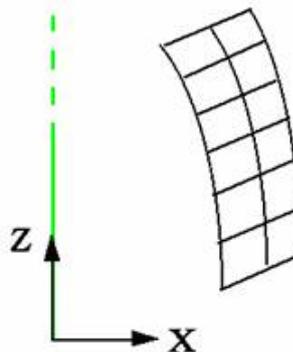
A broken spring on a dial phone, would also cause a mechanism

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The axisymmetric case is special and must be handled carefully. The NASTRAN<sup>®</sup> codes embed axisymmetry into a 3-D space. Other codes restrict the space, typically to the xz plane.

If the element is a special axisymmetric element restricted to the xz plane, there is only one rigid body mode---translation in the z direction.

Hoop stresses prevent both x translation and rotation in the xz plane as rigid body modes.



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## B. RULES OF THUMB

(19)

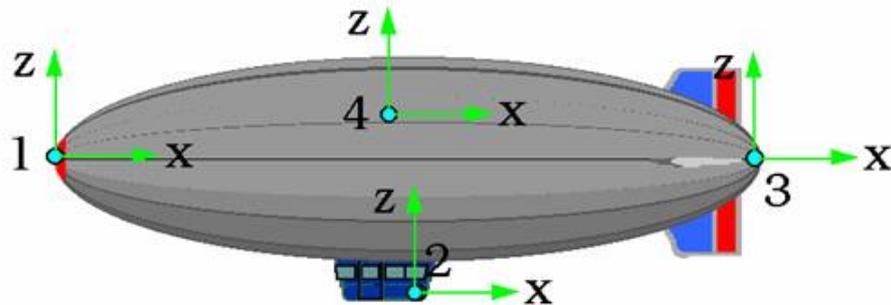
BODY CONSTRAINED TO LIE IN:	SUBCASE	# OF RIGID BODY MODES
3-D		6
2-D	plane stress plane strain	3
	axisymmetry	1
1-D		1

Codes vary widely in how two-dimensional cases are handled. Some codes (SAP) treat 2-D with special elements. Other codes (NASTRAN®) embed plane stress and plane strain into 3-D.

## C. SUPPRESSION OF RIGID BODY MODES

(20)

Consider a vehicle in flight which is to be stress analyzed. The analyst must constrain 6 degrees of freedom to remove 6 rigid body modes. If this is not done, the static analysis will fail!



Node 2 (beam / plate elements):

$$T_1, T_2, T_3, R_1, R_2, R_3 = 0 \quad (\text{easy})$$

Node 1 (solid):	$T_1, T_2, T_3 = 0$	} (good)
Node 3 (solid):	$T_2, T_3 = 0$	
Node 4 (membrane):	$T_2 = 0$	

Node 1 (solid):	$T_1, T_2, T_3 = 0$	} (fails)
Node 3 (solid):	$T_1, T_2, T_3 = 0$	

This last attempt fails because it does not constrain the rolling rigid body mode and it prevents elastic strain from nose to tail! This also "grounds" live loads in the longitudinal direction:



(21)

## EXERCISE 8.1

[D:10] The plate structure shown in Figure E8.1 is loaded and deforms in the plane of the figure. The applied load at  $D$  and the supports at  $I$  and  $N$  extend over a fairly narrow area. Give a list of what you think are the likely “trouble spots” that would require a locally finer finite element mesh to capture high stress gradients.

Identify those spots by its letter and a reason. For example,  $D$ : vicinity of point load.

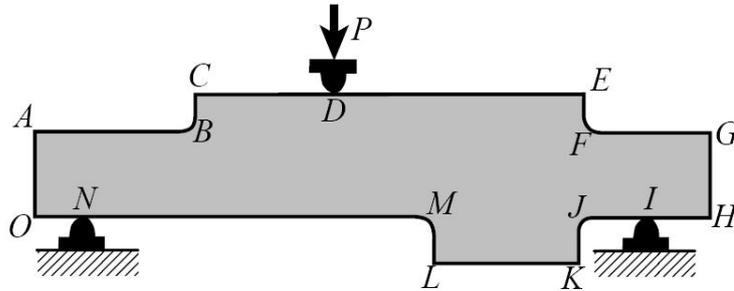


Figure E8.1. Plate structure for Exercise 8.1.

## EXERCISE 8.2

[D:15] Part of a two-dimensional FE mesh has been set up as indicated in Figure E8.2. Region  $ABCD$  is still unmeshed. Draw a *transition mesh* within that region that correctly merges with the regular grids shown, uses 4-node quadrilateral elements (quadrilaterals with corner nodes only), and *avoids triangles*. Note: There are several (equally acceptable) solutions.

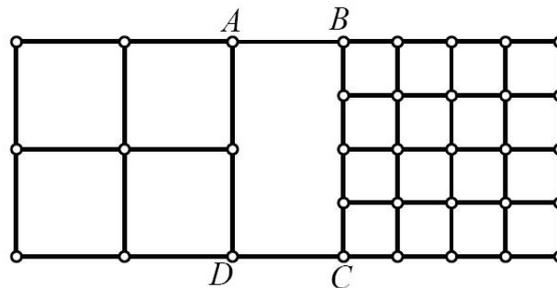


Figure E8.2. Plate structure for Exercise 8.2.

## EXERCISE 8.3

[A:15] Compute the “lumped” nodal forces  $f_1$ ,  $f_2$ ,  $f_3$  and  $f_4$  equivalent to the linearly-varying distributed surface load  $q$  for the finite element layout defined in Figure E8.3. Use both NbN and EbE lumping. For example,  $f_1 = 3q/8$  for NbN. Check that  $f_1 + f_2 + f_3 + f_4 = 6q$  for both schemes (why?). Note that  $q$  is given as a force per unit of vertical length.

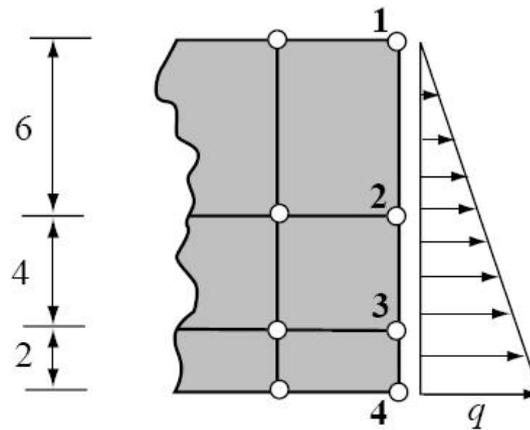


Figure E8.3. Mesh layout for Exercise 8.3.

## EXERCISE 8.4

[D:15] Identify the symmetry and antisymmetry lines in the two-dimensional problems illustrated in Figure E8.4. They are: (a) a circular disk under two diametrically opposite point forces (the famous “Brazilian test” for concrete); (b) the same disk under two diametrically opposite force pairs; (c) a clamped semiannulus under a force pair oriented as shown; (d) a stretched rectangular plate with a central circular hole. Finally (e) and (f) are half-planes under concentrated loads.<sup>6</sup>

Having identified those symmetry/antisymmetry lines, state whether it is possible to cut the complete structure to one half or one quarter before laying out a finite element mesh. Then draw a coarse FE mesh indicating, with rollers or fixed supports, which kind of displacement BCs you would specify on the symmetry or antisymmetry lines. *Note: Do all sketches on your paper, not on the printed figures.*

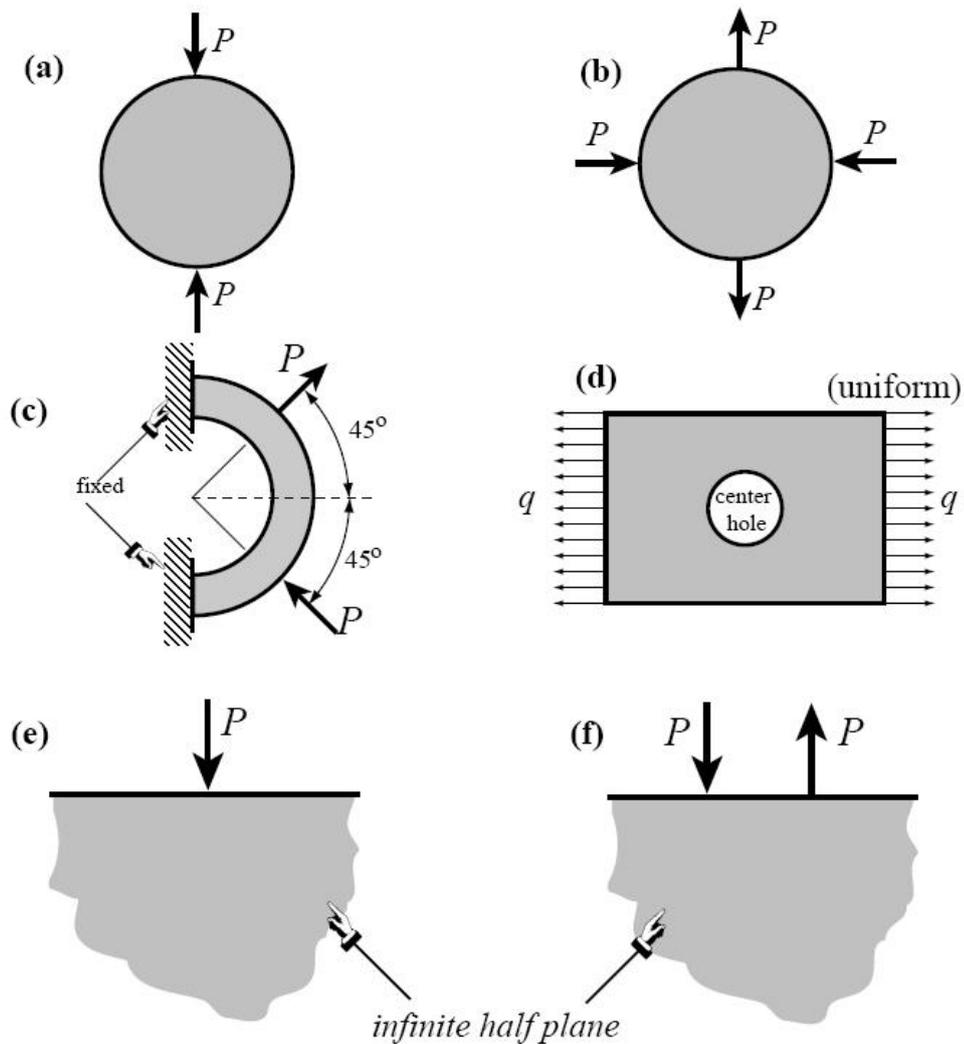


Figure E8.4. Problems for Exercise 8.4.